

Pile-driving analyses of monopiles with pre-installed flanges

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- Introduction Flanged Monopile Connections
- Pile-Driving Analyses
- Flange Loading during Driving
- Concluding Remarks

Monopile Foundations – Grouted vs. Bolted Connection



Grouted Connection Dan Tysk, www.dantysk.vattenfall.de

Grouted connection

- Overlap between TP and MP filled with high-density concrete
- Sophisticated installation with restricted weather window



Bolted Connection Sandbank, www.blog.vattenfall.de

Bolted flange connection

- Steel-to-steel connection with bolted flanges
- Pile-Driving on MP-Flange

TΡ

MP

Pile-Driving on Flanges



Steel-to-steel impact

Ram → Anvil → Flange

30 blows/min, 3000 kJ, 200 MN impact force

Driving-proof flange design Inclination towards inside Load transfer through outer flange



Design Characteristic – Monopile Flange

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Pile-Driving Analyses – Driveability Study



Wave Equation Analysis

Stress wave propagation (1D Wave Equation)

$$\frac{\partial^2 w}{\partial t^2} = c^2 \frac{\partial^2 w}{\partial z^2} \qquad c = \sqrt{\frac{E}{\rho}} \approx 5200 \frac{m}{s}$$

Soil resistance

 Spring-Damper model with in-situ soil parameters (CPT)

Outcome:

- Blow-Count (blows/penetration depth)
- Axial stresses

Validation of FE-Model against **Transient dynamic FE Analysis Wave Equation Analysis** t L/c 0 1 2 GRLWEAP ------LS-DYNA keyword deck by LS-PrePost 200 LS Dyna — Fringe Levels Time = 0 Contours of Z-stress 3.000e-02 min=0, at elem# 1 1.200e-02 max=0, at elem# 1 max displacement factor=100 -6.000e-03 150 -2.400e-02 Impact force top force [MN] -4.200e-02 (pile head) -6.000e-02 100 -7.800e-02 -9.600e-02 -1.140e-01 50 -1.320e-01 -1.500e-01 0 | 35 0 5 10 15 20 25 30 40 time [ms] t L/c 3 0 1 2 GRLWEAP -----0 Settlement settlement [mm] -2 (pile toe) _4 -6 90 -8 -10 10 20 0 30 40 50 time [ms]

Pile-Driving Analyses – Detailed Finite Element Model

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Flange Loading – Hot Spots









(1) Flange surface

- Local plastification at contact edge
- 'Polished ring' visible on-side

(2) Flange neck

- Stress wave propagation through 'bottleneck'
- High stress utilization

Flange Loading – Initiation of Oscillation



Flange Loading – Driving induced Fatigue Damage



30 - 40 % reduction of fatigue strength during pile-driving

Mitigating measures

- Increased wall thickness: $\Delta t = 5 \text{mm} \rightarrow -15 \text{\%}$ operational damage
- Reduced impact energy: $0.75 \cdot E_{Hammer} \rightarrow -10$ % driving damage

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Impact driving on flanges is well feasible, but requires:

- Accurate prediction of pile-driving loads

– Highest accuracy in manufacturing + testing

– Offshore: High precision in driving and quality assurance

Thank you for your attention!

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App. 01 Optimized flange geometry – Design Driver

Flange inclination (g)

Compromise

- Increase g to ensure that loads are only transferred through the defined contact area
- Minimize g to ease offshore installation (gap of MP-TP flange connection closed by tightening of bolts / shim plates)



Major influence on load distribution

- Larger c: Increase of downwards bending
- Smaller c: Increase of upwards bending





Flange neck (a/b)

 Elliptical shape to soften the 'bottle neck' (a/b = 0.5 results in a stress decrease of up to 10% compared to a circular shape)

App. 02 Misalignments/Tolerances

Misalignments of the hammer-pile configuration

- A vertically driven monopile is ensured by a positioning system which adjusts the verticality during the early driving phase.
 Driving at high energy is only performed once the pile reaches a stable vertical position.
- The hammer sleeve fixates the hammer upon the monopile. Therefore, misalignments between anvil and flange top are neglected.

Flatness tolerances of the anvil-flange contact

Flange and anvil: Manufacturing tolerances << 1 mm
Considered by modelling of sinus shaped contact surface



Gripper Arm, www.houlderltd.com



